INCLUSION OF TROPICAL STORMS
FOR THE COMBINED TOTAL STORM TIDE FREQUENCY RESTUDY FOR WALTON COUNTY, FLORIDA
(Revised May 2009)

Sponsored by
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### 1.0 Background

In accordance with the objectives and rationale of the Florida Coastal Construction Control Line, the reestablishment of the line is based on the damage potential of 100 year return period hurricanes. A report entitled "Combined Total Storm Tide Frequency Restudy for Walton County, Florida" (Reference (1)) was submitted to FDEP in November 2007. This additional study is requested by the FDEP to include the tropical storms in the storm surge simulations. Since the methodology and procedures used for this study are the same as for the report mentioned above, only the storm statistics and the results are presented in this report. This report revises the hydrograph in Appendix B from the previous two reports, which were submitted to the FDEP in June 2008 and March 2009, respectively.

### 2.1 Introduction and Data Source

The statistical parameters are based on historical storm data as presented in References (2) and (3). In brief, the empirical cumulative probability distributions are plotted for each of the parameters of interest and are then approximated by a series of straight line segments for computer application. Storm parameters are considered to be independent. The following subsections describe the statistical characteristics of the individual parameters of interest.

### 2.2 Storm Frequency and Direction

The storms causing appreciable storm tides in the vicinity of the Walton County shoreline are classified as "landfalling" or "alongshore" storms. Reasonably good data are available describing the characteristics of the storms impacting the area from 1900 to 2006. For purposes of this report, the data contained in References (2) and (3) that fall within a 300 n . mi. segment of the coast comprising the study area are used. The storm direction is defined here as the azimuth from which the storm is translating at the time of landfall, or, if an alongshore storm, when in close proximity to the site.

For purposes of this study, landfalling storms are considered to be of possible significance if they made landfall within a $300 \mathrm{n} . \mathrm{mi}$. segment of the coast comprising the study area. This segment is extended 200 n . mi. west and 100 n . mi. east from the midpoint of the Walton County shoreline. Accordingly, there were 68 landfalling and 3 alongshore storms occurring in the years

1900 through 2006. The table in Appendix A lists the storms used in this study.
Based on historical data, it is expected that within a 1,000 year period a total of 664 storms will occur within the 300 n . mi. segment of the coast comprising the study area. Of the 664 storms, 636 will be landfalling and 28 alongshore storms.

For purposes of simulation, the cumulative probability distribution of storm track direction $\left(\theta_{\mathrm{N}}\right)$ is presented in Figure 1.


Figure 1 Cumulative Probability Distribution of Storm Track Direction, $\theta_{\mathbf{N}}$

### 2.3 Radius to Maximum Winds and Central Pressure Deficit

The cumulative probability distributions of radius to maximum winds for landfalling and alongshore storms are presented in Figures 2 and 3, respectively. The cumulative probability distributions of pressure deficit for landfalling and alongshore storms is presented in Figure 4.


Figure 2 Cumulative Probability Distribution of Radius to the Maximum Wind, R, for Landfalling Storms


Figure 3 Cumulative Probability Distribution of Radius to the Maximum Wind, R, for Alongshore Storms

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Figure 4 Cumulative Probability Distribution of Central Pressure Deficit, $\Delta \mathrm{p}$

### 2.4 Forward Speed

The cumulative probability distribution of the forward speed of translation for landfalling and alongshore storms is presented in Figure 5.


Figure 5 Cumulative Probability Distribution of Translation Speed, $\mathrm{V}_{\mathrm{F}}$

### 2.5 Track Position

For the landfalling storms, the track position is determined by the y coordinate, $\mathrm{Y}_{\mathrm{F}}$, representing the landfalling point. Figure 6 presents the cumulative probability distribution for the actual landfalling position, $\mathrm{Y}_{\mathrm{F}}$, for landfalling storms. Figure 7 presents the cumulative probability distribution for the actual offshore distance, $\mathrm{X}_{\mathrm{L}}$, for alongshore storms.

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Figure 6 Cumulative Probability Distribution of Landfalling Distance, $\mathrm{Y}_{\mathrm{F}}$, for Landfalling Storms

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$(75,1.00)$


Figure 7 Cumulative Probability Distribution of Offshore Distance, $\mathrm{X}_{\mathrm{L}}$, for Alongshore Storms

### 3.1 Simulation of a n-Year Sequence of Storm Associated Storm Tides

With the statistical characteristics of historical storms available and the two-dimensional model calibrated as described in the preceding section, the simulation shown in Figure 8 is carried out.

The first phase of the simulation comprises the selection of the storm characteristics in accordance with the historical data. In each storm, this involves the following:

1) Quantifying $\Delta p, R, V_{F}, \theta_{N}$ and storm track in accordance with the historical probabilities.
2) For these characteristics, a random astronomical tide from the storm season is generated as a boundary condition to the two-dimensional numerical model and the model is run to determine the storm surge at the site of interest. This storm surge with dynamic wave set up is then adjusted in accordance with the factors obtained from the two-dimensional model calibration runs for the landward grid at each time step to yield the combined total storm tide.
3) Determining whether enough storms have been simulated for the n-year simulation.
4) After the required number of storms and associated storm tides have been simulated, the peak water levels for each storm are ranked and the return period, TR, is calculated, according to

$$
\mathrm{TR}=1000 / \mathrm{M}
$$

where M is the rank of the combined total storm tide level. (For example, since the simulation was carried out for a 1,000 year period, the highest combined total tide level would have a return period of 1,000 years, the fourth highest water level would have a return period of 100 years, etc.). Finally, by presenting these results on semi-log paper, it is possible to interpolate return periods of $5,10,20,40$ and 50 years.


Figure 8 Flow Chart for Two-Dimensional Storm Tide Simulations

### 3.2 Simulation

To summarize information presented earlier, this phase includes the simulation of the occurrence of 1,000 years of storms along a shoreline segment of 300 n . mi. The simulated storms are given directional distributions according to Figure 5. In an average 1,000 year period, there would be a total of 664 storms.

Selection of Storm Parameters - Each of the five idealized storm parameters, [Radius to Maximum Winds, R; Central Pressure, po (or Central Pressure Deficit, $\Delta \mathrm{p}$ ); Track Direction, $\theta_{\mathrm{N}}$; System Forward Speed, $\mathrm{V}_{\mathrm{F}}$; and Track Position] is determined randomly in accordance with the associated cumulative probability distribution functions. The procedure is described below for the track direction, $\theta_{\mathrm{N}}$, and is similar for all other variables.

The approximate piece-wise linear cumulative probability distribution function for track direction, $\theta_{\mathrm{N}}$, is shown in Figure 5 . The nature of this function is such that the predominant directions are those where the function rises steeply. To randomly select a track direction in accordance with the distribution function, the computer first generates a random number between 0 and 1 and then selects the $\theta_{\mathrm{N}}$ corresponding to that cumulative probability. The other four parameters are determined similarly with a separate and independent random number being generated for each parameter and the appropriate cumulative probability distribution used.

Calculation of Storm Surge with the Effect of Astronomical Tide - A particular storm can be "phased" such that the maximum resulting storm surge is increased or decreased by astronomical tidal fluctuations. Considering the predicted ocean astronomical tidal fluctuations at Dog Island West End, Gulf of Mexico from June 1 to November 30, 1984 to be representative of those occurring during the storm season and assuming the phasing of storm occurrence and astronomical tides to be independent, the combination of these tidal components is carried out in the following manner.

With the storm parameters established, a starting time for the storm is selected randomly between June 1 and November 30, 1984. The corresponding astronomical tide at the starting time is generated and varies with time thereafter according to the input astronomical tide data. The calculation of the storm surge history by the calibrated two-dimensional model is thus phased with the astronomical tide to yield the combined storm surge and astronomical tide water level history at the site of interest.

### 3.3 Computation of Return Periods

With a sufficient number (664) of maximum combined total storm tides simulated to represent a typical 1,000 year time interval, the tides associated with various return periods of interest are determined. The 664 maximum combined total storm tides are ranked in descending order with the largest occurring first. The return period, TR, of the ranked tides is then

$$
\mathrm{TR}=1000 / \mathrm{M}
$$

in which

$$
\begin{aligned}
\mathrm{TR}= & \text { Return period in years between expected exceedances of the associated } \\
& \text { maximum storm tide } \\
\mathrm{M}= & \text { Rank of maximum storm tide }
\end{aligned}
$$

As an example, for $\mathrm{M}=664$ (associated with the lowest water level) the return period would be:

$$
\mathrm{TR}_{664}=1000 / 664=1.51 \text { years }
$$

which indicates that the smallest storm tide could be expected to be exceeded approximately once every 2 years. As a second example, the return period for $\mathrm{M}=20$ is

$$
\mathrm{TR}_{20}=1000 / 20=50 \text { years }
$$

The ranked maximum combined total storm tides and associated return periods can be plotted and the combined total storm tide associated with any return period determined. Finally, it is noted that it is possible to run the simulation procedure any number of times to determine the stability (constancy) of any combined total storm tide associated with a given return period. It is expected that for a 1,000 year simulation, the storm tides associated with the longer (> 250 year) return periods would not be well-defined by one simulation and would exhibit variation from simulation to simulation. However, the storm tides associated with the lower return periods (TR < 100 years) should be well-defined by a 1,000 year simulation and hence are not expected to vary significantly for various simulations.

### 4.0 Results

Five 1,000-year simulations for Walton County were carried out employing the computer methods and storm statistics presented in the preceding sections. The combined total storm tides above NGVD and the associated return periods are plotted on semi-log paper in Figure 9 for Walton County. Each data point represents the average value of five simulations and a curve drawn through the data points is adopted to represent the tide-frequency relationship.


Figure 9 Combined Total Storm Tide Elevation Versus Return Period for Walton County

Table I below gives the combined total storm tide values and corresponding return periods for Walton County.

Table I
Combined Total Storm Tide Levels (ft.) for Various Return Periods

| Return <br> Period, <br> TR (years) | West <br> Profile <br> NGVD29 | West <br> Profile <br> NAVD88 | Middle <br> Profile <br> NGVD29 | Middle <br> Profile <br> NAVD88 | East <br> Profile <br> NGVD29 | East <br> Profile <br> NAVD88 |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| 50 | 9.5 | 9.1 | 9.3 | 8.9 | 9.5 | 9.1 |
| 40 | 8.9 | 8.5 | 8.7 | 8.3 | 9.0 | 8.6 |
| 25 | 8.1 | 7.7 | 7.9 | 7.5 | 8.1 | 7.7 |
| 20 | 7.6 | 7.2 | 7.5 | 7.1 | 7.7 | 7.3 |
| 15 | 7.0 | 6.6 | 6.9 | 6.5 | 7.0 | 6.6 |
| 10 | 5.9 | 5.5 | 6.0 | 5.6 | 6.1 | 5.7 |
| 5 | 4.1 | 3.7 | 4.2 | 3.8 | 4.2 | 3.8 |

*Includes contributions of: wind stress, barometric pressure, dynamic wave set-up and astronomical tide.

These results are not intended to replace the storm surge information in the report, "Combined Total Storm Tide Frequency Restudy for Walton County, Florida" (Reference (1)). Based on the actual storm event data by Leadon (Reference (4)), hydrographs for return periods of 15 and 25 years are selected from simulated storms and are presented in Appendix B. A minor adjustment of the combined total storm tide hydrograph may be required such that the peak corresponds to the combined total storm tide level provided in Table I for each specific case.

## REFERENCES

1. Wang, S., Manausa, M., Dean, R. and Walton, T., "Combined Total Storm Tide Frequency Restudy for Walton County, Florida", Beaches and Shores Resource Center, Florida State University, November 2007.
2. U. S. Department of Commerce, National Oceanic and Atmospheric Administration, "Storm Climatology for the Atlantic and Gulf Coasts of the United States", NOAA Technical Report NWS 38, April 1987.
3. U. S. Department of Commerce, National Oceanic and Atmospheric Administration, "Storm Best Track Files (HURDAT), 1851 - 2005", http://www.nhc.noaa.gov.
4. Leadon, M., "Evaluation of Storm Tide Measurements at Panama City Beach, Florida 1993-2007", Beaches and Shores Resource Center, Florida State University, May 2009.

## APPENDIX A

SUMMARY OF HISTORICAL STORMS AFFECTING WALTON COUNTY

| \# | Date | Name | $\begin{gathered} \theta_{\mathrm{N}} \\ \text { (deg.) } \end{gathered}$ | $\begin{gathered} \mathrm{Y}_{\mathrm{F}} \\ \text { (n.mi.) } \end{gathered}$ | $\begin{gathered} \mathrm{V}_{\mathrm{F}} \\ \text { (knots) } \end{gathered}$ | $\begin{gathered} \Delta \mathrm{p} \\ \text { (in.Hg) } \end{gathered}$ | $\begin{gathered} \mathrm{R} \\ \text { (n.mi.) } \end{gathered}$ | Type |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| 1 | 8/ 4/1901 |  | 214.6 | 121.93 | 6.08 | -1.18 |  | L |
| 2 | 9/ 9/1903 |  | 156.6 | -29.86 | 8.71 | -0.97 |  | L |
| 3 | 9/19/1906 |  | 165.4 | 147.17 | 10.33 | -1.63 |  | L |
| 4 | 8/8/1911 |  | 127.6 | 128.9 | 6.54 | -0.74 |  | L |
| 5 | 9/10/1912 |  | 166.1 | 128.1 | 7.21 | -0.74 |  | L |
| 6 | 8/31/191 |  | 183.0 | -42.9 | 16.02 | -0.85 |  | L |
| 7 | 6/29/1916 |  | 151.6 | 178.89 | 9.09 | -1.86 | 26 | L |
| 8 | 10/12/191 |  | 189.7 | 59.94 | 20.3 | -1.15 | 19 | L |
| 9 | 9/21/191 |  | 229.0 | 20.05 | 9.15 | -1.39 | 33 | L |
| 10 | 9/13/192 |  | 235. | -27.64 | 5.26 | -0.64 |  | L |
| 11 | 9/11/192 |  | 124. | 45.1 | 4.57 | -1.83 | 17 | AL |
| 12 | 9/22/192 |  | 189.7 | -51.29 | 5.07 | -0.64 |  | L |
| 13 | 8/26/193 |  | 132.0 | 160.37 | 10.46 | -0.74 |  | L |
| 14 | 7/27/193 |  | 144.2 | 28.21 | 7.39 | -1.18 | 19 | L |
| 15 | 8/ 7/1939 |  | 127.6 | -13.31 | 6.54 | -0.74 |  | L |
| 16 | 10/3/1941 |  | 175.5 | -82.81 | 11.03 | -0.85 | 18 | L |
| 17 | 8/20/1950 | BAKER | 190.4 | 87.78 | 14.24 | -0.85 | 21 | L |
| 18 | 9/23/195 | FLORENCE | 220. | -0.85 | 9.25 | -0.97 |  | L |
| 19 | 9/21/1956 | FLOSSY | 247.1 | 12.5 | 10.3 | -0.98 | 18 | L |
| 20 | 9/14/1960 | ETHEL | 18 | 152.59 | 8 | -0.95 | 22 | L |
| 21 | 6/ 4/1966 | ALMA | 215.7 | -93.62 | 7.39 | -0.95 | 20 | L |
| 22 | 8/14/1969 | CAMILLE | 161.6 | 198.18 | 13.7 | -3.07 | 18 | L |
| 23 | 6/14/197 | AGNES | 200.9 | -44.3 | 9.64 | -0.89 | 20 | L |
| 24 | 9/13/197 | ELOISE | 190.4 | 4.21 | 28.47 | -1.72 | 18 | L |
| 25 | 8/29/1979 | FREDERIC | 158.5 | 129.9 | 11.82 | -1.98 | 33 | L |
| 26 | 8/28/198 | ELENA | 103.9 | 50.65 | 12.45 | -1.77 | 16.2 | AL |
| 27 | 10/26/198 | JUAN | 201.9 | 72.08 | 16.17 | -0.92 |  | L |
| 28 | 11/15/198 | KATE | 220. | -52.08 | 13.21 | -1.36 | 18.6 | L |
| 29 | 6/3/1995 | ALLISON | 215.2 | -92.7 | 13.47 | -0.68 |  | L |
| 30 | 7/31/199 | ERIN | 122.7 | 39.78 | 9.24 | -1.01 | 19.5 | AL |
| 31 | 9/27/1995 | OPAL | 201.2 | 44.1 | 21.46 | -2.22 | 46.9 | L |
| 32 | 7/16/1997 | DANNY | 220. | 75.98 | 2.78 | -0.86 | 13 | L |
| 33 | 8/31/1998 | EARL | 230.7 | -31.97 | 18.96 | -0.77 |  | L |
| 34 | 9/15/1998 | GEORGES | 163.9 | 160.44 | 6.24 | -1.45 | 17 | L |
| 35 | 9/2/2004 | IVAN | 187.0 | 86.92 | 14.11 | -2.07 | 24 | L |
| 36 | 7/ 4/2005 | DENNIS | 156.6 | 58.14 | 17.43 | -2.1 | 14 | L |
| 37 | 9/11/1900 |  | 214.6 | 113.53 | 6.08 | -0.18 |  | L |
| 38 | 6/11/1901 |  | 175.0 | -84.12 | 10.04 | -0.18 |  | L |
| 39 | 9/9/1901 |  | 222.9 | 20.06 | 17.75 | -0.38 |  | L |
| 40 | 9/21/1901 |  | 185.4 | -87.49 | 18.08 | -0.24 |  | L |
| 41 | 10/3/1902 |  | 205. | 51.18 | 17.79 | -0.38 |  | L |
| 42 | 10/31/190 |  | 238.6 | 16.21 | 19.21 | -0.18 |  | L |
| 43 | 6/8/1906 |  | 18 | -32.04 | 11 | -0.31 |  | L |
| 44 | 6/24/1907 |  | 255.8 | -15.82 | 20.47 | -0.38 |  | L |
| 45 | 9/18/1907 |  | 189.7 | 141.31 | 5.07 | -0.24 |  | L |
| 46 | 9/27/1907 |  | 236.7 | -32.31 | 23.73 | -0.31 |  | L. |
| 47 | 7/2/1919 |  | 166.1 | 49.04 | 7.21 | -0.38 |  | L |


| $\#$ | Date | Name | $\theta_{\mathrm{N}}$ <br> (deg.) | $\mathrm{Y}_{\mathrm{F}}$ <br> (n.mi.) | $\mathrm{V}_{\mathrm{F}}$ <br> (knots) | $\Delta \mathrm{p}$ <br> (in.Hg) | R <br> (n.mi.) | Type |
| :---: | ---: | :--- | ---: | ---: | ---: | ---: | ---: | :--- |
| 48 | $10 / 12 / 192$ |  | 152.6 | 95.27 | 5.63 | -0.24 |  | L |
| 49 | $10 / 16 / 192$ |  | 177.9 | 153.72 | 24.02 | -0.31 |  | L |
| 50 | $8 / 7 / 1928$ |  | 154.3 | -77.91 | 9.98 | -0.31 |  | L |
| 51 | $10 / 1 / 1934$ |  | 203.3 | 92.97 | 6.53 | -0.18 |  | L |
| 52 | $6 / 12 / 1939$ |  | 163.9 | 106.7 | 6.24 | -0.18 |  | L |
| 53 | $9 / 9 / 1944$ |  | 212.3 | 143.41 | 17.75 | -0.18 |  | L |
| 54 | $9 / 7 / 1947$ |  | 129.0 | 200 | 11.11 | -0.18 |  | L |
| 55 | $7 / 7 / 1948$ |  | 203.3 | -2.63 | 6.53 | -0.18 |  | L |
| 56 | $5 / 25 / 1953$ | ALICE | 18 | -21.61 | 6 | -0.24 |  | L |
| 57 | $6 / 8 / 1957$ |  | 222.3 | -93.29 | 24.35 | -0.18 |  | L |
| 58 | $9 / 7 / 1957$ | DEBBIE | 224.6 | 9.64 | 9.83 | -0.18 |  | L |
| 59 | $10 / 6 / 1959$ | IRENE | 209.9 | 60.82 | 10.38 | -0.36 |  | L |
| 60 | $9 / 17 / 1960$ | FLORENCE | 143. | 78.41 | 8.71 | -0.03 |  | L |
| 61 | $6 / 11 / 1965$ |  | 226.3 | -4.29 | 20.28 | -0.31 |  | L |
| 62 | $9 / 29 / 1969$ |  | 18 | 16.83 | 15 | -0.5 |  | L |
| 63 | $7 / 19 / 1970$ | BECKY | 190.8 | -60.71 | 18.33 | -0.12 |  | L |
| 64 | $6 / 30 / 1994$ | ALBERTO | 203.3 | 14.1 | 8.71 | -0.59 |  | L |
| 65 | $8 / 14 / 1994$ | BERYL | 212.9 | -37.77 | 4.76 | -0.39 |  | L |
| 66 | $9 / 15 / 200$ | HELENE | 207.3 | 23.9 | 11.26 | -0.36 |  | L |
| 67 | $8 / 2 / 2001$ | BARRY | 175.5 | 10.33 | 11.03 | -0.62 |  | L |
| 68 | $9 / 12 / 2002$ | HANNA | 220. | 97.32 | 10.57 | -0.3 |  | L |
| 69 | $8 / 3 / 2004$ | BONNIE | 236.4 | -88.92 | 21.74 | -0.33 |  | L |
| 70 | $6 / 8 / 2005$ | ARLENE | 176. | 73.64 | 13.03 | -0.65 |  | L |
| 71 | $7 / 3 / 2005$ | CINDY | 218.3 | 136.5 | 15.3 | -0.56 |  | L |

Landfalling Storms $=68$; Alongshore Storms $=3$; Exiting Storms $=0$
${ }^{1}$ Values are estimated prior to landfall.

## APPENDIX B

## COMPUTED 15 AND 25 YEAR HYDROGRAPHS <br> FOR WALTON COUNTY




